

Seismic Analysis of Multi-Storied Building with Shear Walls Using ETABS-2013

N. Janardhana Reddy¹, D. Gose Peera², T. Anil Kumar Reddy³

^{1,2,3}Bheema institute of Technology, Alur Road, Adoni 518301, Kurnoll dist

Abstract: Shear walls are structural members used to elongate the strength of R.C.C. structures. These shear walls will be construct in each level of the structure, to form an effective box structure. Equal length shear walls are placed symmetrically on opposite sides of outer walls of the building. Shear walls are added to the building interior to provide more strength and stiffness to the building when the exterior walls cannot provide sufficient strength and stiffness. It is necessary to provide these shear walls when the tolerable span-width ratio for the floor or roof diaphragm is exceeded. The present work deals with a study on the improvement location of shear walls in symmetrical high rise building. Position of shear walls in symmetrical buildings has due considerations. In symmetrical buildings, the center of gravity and center of rigidity coincide, so that the shear walls are placed symmetrically over the outer edges or inner edges (like box shape). So, it is very necessary to find the efficient and ideal location of shear walls in symmetrical buildings to minimize the torsion effect. In this work a high rise building with different places of shear walls is considered for analysis. The multi storey building with 14 storeys is analyzed for its displacement, strength and stability using ETABS-2013 software. For the analysis of the building for seismic loading with two different Zones (Zone-II & Zone-V) is considered with a soil I & soil III types. The analysis of the building is done by using equivalent static method and dynamic method. The results from the analysis obtained from both the methods are presented in tabular form and the results are compared using graphical form

Keywords: ETABS-2013, SHEAR WALL, IS 456-2000, IS1893-2002

1. Introduction

Adequate stiffness is to be ensured in high rise buildings for resistance to lateral loads induced by wind or seismic events. Reinforced concrete shear walls are designed for buildings located in seismic areas, because of their high bearing capacity, high ductility and rigidity. In high rise buildings, beam and column dimensions work out large and reinforcement at the beam-column joints are quite heavy, so that, there is a lot of clogging at these joints and it is difficult to place and vibrate concrete at these places which does not contribute to the safety of buildings. These practical difficulties call for introduction of shear walls in High rise buildings.

1.1 Structural forms

Lateral loads can develop high stresses, produce sway movement or cause vibration. Therefore, it is very important to have sufficient strength for the structure against vertical loads. Earthquake and wind forces are the only major lateral forces that affect the buildings. The function of lateral load resisting systems or structure form is to absorb the energy induced by these lateral forces by moving or deforming without collapse. The determination of structural form of a tall building or high rise building would perfectly involve only the arrangement of the major structural elements to resist most efficiently the various combinations of lateral loads and gravity loads. The internal planning

1. The material and the method of construction
2. The nature and magnitude of the horizontal loading
3. The external architectural treatment
4. The height and proportions of the building and
5. The planned location and routing of the service systems

The taller and more the slender a structure, the more important the structural factors become and the more necessary it is to choose an appropriate structural form or the lateral loading system for the building. In high rise buildings which are designed for a similar purpose and of the same height and material, the efficiency of the structures can be compared by their weight per unit floor area.

1.2 Factors affecting earthquake design of structure

- 1) Natural frequency of the building
- 2) Damping factor of the structure
- 3) Type of foundation of the structure
- 4) Importance of the building
- 5) Ductility of the structure

Quite a few methods are available for the earthquake analysis of buildings; two of them are presented here:

- a. Equivalent Static Lateral Force Method (pseudo static method).
- b. Dynamic analysis.
 - (1) Response spectrum method.
 - (2) Time history method.

2. Required Indian Standard Codes

IS 456:2000 As per clause 32, design for wall describes, design of horizontal shear in clause 32.4 given details of how shear wall have to be constructed.

IS 1893-2002 Criteria of Earth Quake resistant Buildings Part (3) page23, clause4.2 gives the estimation of earth quake loads.

IS 13920:1993 it gives the ductile detailing of shear wall as per clause9, where 9.1 gives general requirements,9.2 shear

strength 9.3 give flexural strength 9.6 give openings in shear walls.

Ductile detailing, as per the code IS: 13920:1993 is considered very important as the ductile detailing gives the amount of reinforcement required and the alignment of bars.

3. Shear Wall

A shear wall is a wall that is used to resist the shear, produced due to lateral forces. Many codes made the shear wall design for high rise buildings a mandatory. Shear walls are provided when the centre of gravity of building area and loads acted on structure differs by more than 30%. To bring the centre of gravity and centre of rigidity in range of 30%, concrete walls are provided i.e. lateral forces may not increase much. These shear walls start at foundation level and extend throughout the building height. The thickness of the shear wall may vary from 150mm to 400mm. Shear walls are oriented in vertical direction like wide beams which carry earthquake loads downwards to the foundation and they are usually provided along both width and length of the buildings. Shear walls in structures located at high seismic regions require special detailing. The construction of shear walls is simple, because reinforcement detailing of walls is relatively straight forward and easy to implement at the site. Shear walls are effective both in construction cost and effectiveness in minimizing earthquake damage to the structural and non structural elements also.

3.1 Shapes or Geometry of Shear Walls

Shear walls are rectangle in cross section, i.e. one dimension is much larger than the other. While rectangular cross-section is frequent, L- and U-shaped sections are also used. Thin-walled hollow RC shafts around the elevator core of the structure also act as shear walls, and should be taken advantage of to resist earthquake forces.

The Shear Wall sections are classified as six types.

- (a) Box Section
- (b) L – Section
- (c) U - Section
- (d) W – Section
- (e) H - Section
- (f) T – Section

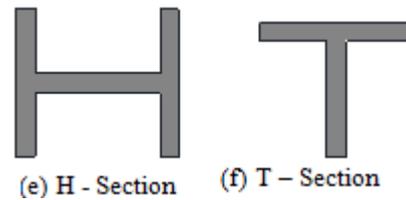
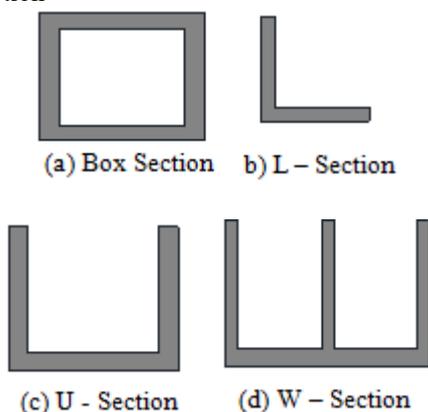


Figure 5.1: Different shapes or geometries of shear walls

3.2 Classification according to behaviour

Shear walls can also be classified according to their behaviour also, they are as follows:

- a) *Shear-shear walls* in which strength and deflection are controlled by shear. These types of shear walls are usually constructed in low rise buildings.
- b) *Ordinary moment shear walls* in which deflection and strength are controlled by means of flexure. These are generally used in high rise buildings to resist high winds and cyclones.
- c) *Ductile moment shear walls* are special walls meant for seismic regions and which have good energy dissipation characteristics under reversed cyclic loads.

3.3 Components of Shear Walls

Reinforced concrete and reinforced masonry shear walls are seldom-simple walls which resist the lateral forces. Whenever a wall has doors, windows, or other openings, the wall must be considered as an assemblage of relatively flexible components like column segments and wall piers and relatively stiff elements like wall segments.

- a) **Column segments:** A column segment is a vertical member whose height exceeds three times its thickness and whose width is less than two and one-half times its thickness. Its load is usually mainly axial. Although it may contribute little to the lateral force resistance of the shear wall its rigidity must be considered. When a column is built integral with a wall, the portion of the column that project from the face the wall is called a pilaster. Column segments shall be designed according to ACI 318 for concrete.
- b) **Wall piers:** A wall pier is a segment of a wall whose horizontal length is between two and one-half and six times its thickness whose clear height is at least two times its horizontal length.
- c) **Wall segments:** Wall segments are components of shear wall that are longer than wall piers. They are the primary resisting components in the shear wall.

Important features in planning and design of shear walls: For all high rise buildings, the problem of providing adequate stiffness and preventing large displacements, are as important as providing adequate strength. Thus shear wall system has two distinct advantages over a frame system.

4. Lateral Forces

The design of multi-storey structures is governed by lateral load resistance requirements in addition with gravity loads. The magnitude of the lateral force on a structure is not only dependent on the acceleration of the ground but also it

depends on the type of the structure. The term lateral loads describes the effect of seismic and wind forces, even though in the recent past it included any horizontal applied forces, this terminology seeks to differentiate lateral loads from the downward acting gravity loads, even though in reality the seismic and wind forces can act in both vertical and horizontal directions. Modern building codes require that wind be applied perpendicular to roof surfaces nearly upward for shallow roofs and that a percentage of earthquake loading be applied vertically.

5. Methodology

5.1 Design Aspect

Earthquakes can occur on both land and sea, at any place on the surface of the earth where there is a major fault. When earthquake occurs on land it affects the man made structure surrounding its origin leading to human lose. When a major earthquake occurs underneath the ocean or sea, it not only affects the structures near it, but also produces large tidal waves known as Tsunami, thus affecting the places far away from its origin. All the structures are designed for the combined effects of gravity loads and seismic loads to verify that sufficient vertical and lateral strength and stiffness are achieved to satisfy the structural concert and acceptable deformation levels prescribed in the governing building code. Because of the innate factor of safety used in the design specifications, most structures tend to be adequately protected against vertical shaking. Vertical acceleration should also be considered in structures with large spans, those in which stability for design, or for overall stability analysis of structures.

1. Serviceability limit state:

The structure undergoes little or no structural damage in this case. Important buildings such as hospitals, atomic power stations, places of assembly etc, which affects a community, should be designed for elastic behavior under expected earthquake forces. These types of structures should be serviceable even after the occurrence of earthquake or cyclones.

2. Damage controlled limit state:

In this case, if an earthquake or cyclone occurs, there can be some damage to the structure but it can be repaired even after the occurrence of the disaster. Most of the permanent buildings should come under this category, so, the structure should be designed for limited ductility response only.

3. Survival limit state:

In this case, the structure is allowed to be damaged in the event of earthquake or cyclone disasters. But, the supports should stand and support the permanent loads coming on to it so that there should be no caving in of the structure and no loss of life. Limited ductile response is cheaper and full ductile response is cheapest. The full ductile detailing is achieved by the theory of plastic hinge formation and also by careful ductile detailing. The current design practice is to construct the structures for the first two limit states as the other is under development stage.

5.2 Design approach in IS 1893 (2002)

The title of IS 1893-2002 is "Criteria for earthquake resist design of structures" and part 1 of this code deals with General Provisions and buildings [1]. According to this code we consider the following magnitudes of earthquakes:

- a) Design basic earthquake (DBE): It is the earthquake which occurs reasonably at least once during the designed life of the structure.
- b) Maximum considered earthquake (MCE): This is the most severe earthquake that can occur in that region as considered by the code. It is divided by factor 2 to get design basic earthquake.

The value of Z, the seismic zone factor given in the code relates the realistic values of effective peak ground acceleration considering MCE and the service life of the structure. The following principles are the basis for the design approach recommended by IS 1893-2002.

- 1) The structure should have the strength to withstand minor earthquakes less than DBE without any damage.
- 2) The structure should able to resist earthquakes equal to DBE without significant damage though some non structural damage may occur
- 3) The structure should able to withstand an earthquake equal to MCE without collapse so that there is no loss of life

As, the actual forces will be much larger than the design forces specified by the code, the ductility arising from the inelastic material behavior and detailing along with the reserve strength are relied upon to account for the difference in the actual and the design lateral loads. Conceptual representation of earthquake resistant design philosophy is depicted in the following figure;

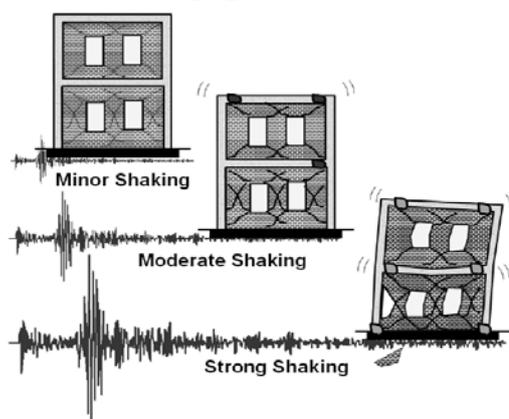


Figure 7.1: Schematic diagram depicting earthquake resistant design philosophy for different levels shaking

5.3 Equivalent Static Method

The equivalent static method of finding lateral forces is also known as the static method or the seismic coefficient method. This method is the simplest one and it requires less computational attempt and is based on formulae given in the code of practice. In all the methods of analyzing a multi storey buildings recommended in the code, the structure is treated as discrete system having concentrated masses at floor levels which comprise the weight of columns and walls in any storey should be equally distributed to the floors above

and below the storey. In addition, the suitable amount of imposed load at this floor is also lumped with it. It is also assumed that the structure flexible and will deflect with respect to the position of foundation; the lumped mass system reduces to the solution of a system of second order differential equations. These equations are formed by distribution of mass and stiffness in a structure, together with its damping characteristics of the ground motion.

5.4 Design Seismic Base Shear

The design seismic base shear or total design lateral force (V_B) along any principal direction shall be determined by the following expression:

$$V_B = A_h \times W$$

Where,

A_h = Design horizontal acceleration spectrum value using the fundamental natural period „T“ in the considered direction of vibration

W = seismic weight of the building

$$A_h = \frac{Z I S_A}{2 R g}$$

The A_h shall be determined by the following expression:

Provided that for any building with T less than 0.1s, the value of A_h shall not be taken less than Z/2 whatever be the value of I/R.

Where,

Z = Zone factor is determined from the following table

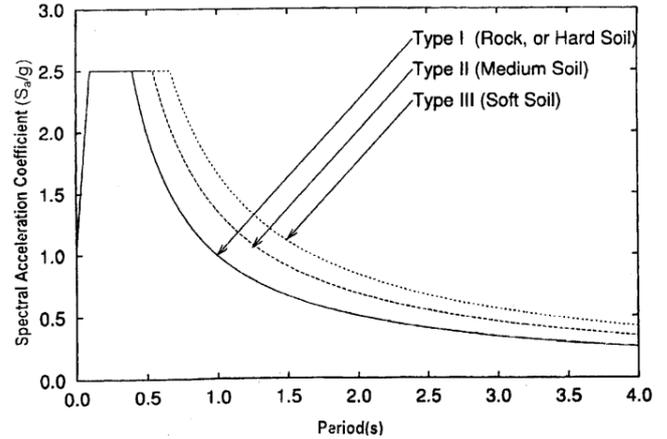
Seismic Zone	II	III	IV	V
Seismic intensity	Low	Moderate	Severe	Very severe
Z	0.10	0.16	0.24	0.36

Zone factor given in the above table is for the Maximum Considered Earthquake (MCE) and service life of structure in a zone. The factor 2 in the denominator of Z is used so as to reduce the Maximum Considered Earthquake (MCE) zone factor to the factor for Design Basis Earthquake (DBE).

I = I represents the importance factor and it depends upon the functional use of the structures. It is characterized by hazardous consequences of its failure, post earthquake functional needs, historical value or economic importance. 1.5 is considered for the important structures like hospitals, schools, monumental buildings etc. and the rest of the buildings it is taken as 1.

R = It is Response reduction factor which depends on the perceived seismic damage performance of the structure, characterized by ductile or brittle deformations of the structure. This ration should not be greater than one. The values for R are given in Table 7 of IS: 1893. The value for R varies between 3 and 5 with respect to ductile reinforcement detailing.

S_a/g = Average response acceleration coefficient as per clause 6.4.5 of IS 1893:2002 as given by below figure and it is based on the damping and the natural periods of the structures.



Note: The value of A_h will not be taken less than 1/2 where ever the value of I/R.

5.5 Time Period

The approximate fundamental natural period of vibration T_a in seconds, of a moment resisting frame building without brick infill panels may be estimated by the following empirical formula

$$T_a = 0.075h^{0.75} \text{ For RC frame building}$$

$$T_a = 0.085h^{0.75} \text{ For steel frame building}$$

The approximate fundamental natural period of vibration in seconds of all other, buildings including moment resisting frame buildings with brick infill panels may be estimated by the following expression.

$$T = \frac{0.09 H}{\sqrt{d}}$$

Where,

H = Height of building in meters (This excludes the basement storey, where basement walls are connected with the ground floor deck or fitted between the columns. But, it includes the basement storey, when they are not connected).

D = Base dimensions of the building at the plinth level, in m, along the considered direction of the lateral force.

5.6 Seismic Weight

The seismic weight of a structure is the sum of seismic weight of all the floors in the structure. The seismic weight of every floor is the sum of its full dead load and appropriate amount of imposed load, the latter being that element of the imposed loads that may sensibly be expected to be attached to the structure at the time of earthquake movement. It includes the weight of permanent and movable partitions, permanent equipment, a part of the live load, etc. While computing the seismic weight of walls and columns in any storey shall be equally distributed to the floors above and below the storey.

5.7 Distribution of Design Force

The computed base shear is now distributed along the height of the building. The shear force, at any level depends on the mass at that level and tends to deform the shape of the structure. Earth quake forces deflect the structure into number of shapes known as the natural mode shapes and the

number of natural mode shapes depends up on the degree of freedom of the system. Generally a structure has continuous system with infinite degree of freedom. The magnitude of the lateral force at a particular floor depends on the mass of the node, the distribution of stiffness over the height of the structure and the nodal displacement in the given mode.

The design base shear (V_B) computed by using the above expression shall be distributed along the height of the building as per the following expression:

$$Q_i = V_B \frac{W_i h_i^2}{\sum_{j=1}^n W_j h_j^2}$$

Where,

Q_i = Design lateral force at floor i.

W_i = Seismic weight of floor i.

H_i = Height of floor i measured from base, and

n = Number of stores in the building i.e., the number of levels at which the masses are located.

The distribution suggested in the code gives parabolic distribution of seismic forces such that seismic shears are higher near top storey for the same base shear. The assumptions involved in the static procedure reflected in the expression are

- a) fundamental mode of the building makes the most significant contribution to base shear, and
- b) The total building mass is considered as against the modal mass that would be used in a dynamic procedure.

The mass and stiffness are evenly distributed in the building.

5.8 Dynamic Analysis

Dynamic analysis shall be carried out to obtain the design seismic force, and its distribution in different levels along the height of the building, and in the various lateral loads resisting element, for the following buildings:

Regular buildings: Those greater than 40m in height in zones IV and V, those greater than 90m in height in zone II and III.

Irregular buildings: All framed buildings higher than 12m in zones IV and V, and those greater than 40m in height in zones II and III.

The analysis of model by dynamic analysis of buildings with unusual configuration should be such that it sufficiently models the types of irregularities present in the building configuration. Buildings with plan irregularities, as defined in Table 4 of IS code: 1893-2002 cannot be modeled for dynamic analysis.

Dynamic analysis may be performed either by the TIME HISTORY METHOD or by the RESPONSE SPECTRUM METHOD,

5.9 Time History Method

The usage of this method shall be on an appropriate ground motion and shall be performed using accepted principles of

dynamics. In this method, the mathematical model of the building is subjected to accelerations from earthquake records that represent the expected earthquake at the base of the structure.

6. Numerical Modeling and Analysis

6.1 Geometrical Properties

- 1) Height of typical storey = 3 m
- 2) Height of ground storey = 3.2 m
- 3) Length of the building = 35 m
- 4) Width of the building = 35 m
- 5) Span in X- direction = 5 m
- 6) Span in Y-direction = 5 m
- 7) Height of the building = 43.7 m
- 8) Number of stores = 14
- 9) Brick wall thickness = 230 mm
- 10) Slab Thickness = 120 mm
- 11) Grade of the concrete = M30
- 12) Grade of the steel = Fe415
- 13) Thickness of shear wall = 230 mm
- 14) Column sizes = 0.7 m X 0.45 m 1th story 0.6m X 0.45 m, 2nd to 9th storey 0.45m X 0.3 m 10th to 14th storey
- XV. Beam sizes = 0.3 m X 0.45 m 1th to 9th storey 0.3 m X 0.3 m 10th to 14th storey

6.2 Loads

1. Live load

Live load from 1st floor to 14th floor = 2 kN/m²
 Live load on 14th floor = 1.5 kN/m²

2. Dead load

Dead load is taken as prescribed by the IS: 875 -1987 (Part-I) [3] Code of Practice Design Loads (other than earthquake) for Buildings and structure.
 Unit weight of R.C.C. = 25 kN/m³
 Unit weight of brick masonry = 19 kN/m³
 Floor finish = 1.5 kN/m²
 Water proofing = 2 kN/m² on terrace roof
 Wall load = 13.8 kN/m on all floors
 Expect terrace Roof = 6.9 kN/m on terrace roof

3. Seismic Loading

In the present work the building is located in Hyderabad which comes under -zone-II and Jammu Kashmir comes under Zone V, using the IS 1893 (Part-I) – 2002(1) the following are the various values for the building considered.

a. Zone factor (Z):

It is a factor to obtain the design spectrum depending on (lie perceived maximum seismic risk characterized by Maximum considered Earthquake (MCE) in the zone in which the structure is located. The basic zone factors included in this standard are reasonable estimate of effective peak ground acceleration.

Zone factor = 0.10 (Zone-II) (from IS 1893 (Part-I)-2002, Table.- 2).

Zone factor = 0.36 (Zone-V) (from IS 1893 (Part-I)-2002,

Table.- 2).

b. Response reduction factor I:

It is the factor by which the actual base-shear force that would be generated if the structure were to remain elastic during its response to the Design Basis Earthquake (DBE) shaking, shall be reduced to obtain the design lateral force.

Response reduction factor = 5.0 (from IS 1893 (Part-1)-2002, Table-7. I

c. Importance factor (I):

It is a factor used to obtain the design seismic force depending on the functional use of the structure, characterized by hazardous consequences of post-earthquake functional need, historical value, or economic importance.

Importance factor (1) = 1 (from IS 1893-2002 (Part-I), Table-6

d. Soil Type

Soil site factor (1 for hard soil, 2 for medium soil, and 3 for soft soil) depending on type of soil average response acceleration coefficient S_a/g is calculated corresponding to 5% damping Refer Clause 6.4.5 of IS 1893-2002. In the present work three type of soil are used. Soil type considered is medium soil, factor 2.

e. Damping

The effect of internal friction, imperfect elasticity of material, slipping, sliding etc in reducing the amplitude of vibration and is expressed as a percentage critical damping.

Damping – 5%

6.3 Material Properties

Modulus of Elasticity

The modulus of elasticity is primarily influenced by the elastic properties of the aggregate and to a lesser extent by the condition of curing and age of the concrete, the mix proportions and the type of cement. The modulus of elasticity is normally related to the compressive strength of concrete.

The modulus of elasticity of reinforced concrete members can be assumed as

$$E = 5000 \sqrt{f_{ck}} \text{ Eq, (17) [3] from IS 456:2000}$$

Where,

E=modulus of elasticity in kN/m^2

f_{ck} = Characteristic cube compressive strength of concrete in N/mm^2

The modulus of elasticity of reinforced concrete members for M_{30} grade concrete is

$$E = 27386.127 \text{ KN/m}^2.$$

Poisson's Ratio: *Poisson's* ratio is the ratio between lateral strains to the longitudinal strain. It is generally denoted by the letters for normal concrete the value of *Poisson's* ratio lies in the range of 0.15 to 0.20 when actually determined from strain measurement. For the present work *Poisson's* ratio is assumed as 0.2 for reinforced concrete.

6.4 Load Combinations

The following Load Combinations have been considered for the design

1. (DL+ LL)
2. (DL ± EQX)
3. (DL ± EQY)
4. (DL + LL ± EQX)
5. (DL + LL ± EQY)
6. (DL ± WLX)
7. (DL ± WLY)
8. (DL + LL ± WLX)
9. (DL + LL ± WLY)

The following Load Combinations have been considered for the design

10. 1.5(DL+ LL)
11. 1.5(DL ± EQX)
12. 1.5(DL ± EQY)
13. 1.2(DL + LL ± EQX)
14. 1.2(DL + LL ± EQY)
15. 1.5(DL ± WLX)
16. 1.5(DL ± WLY)
17. 1.2(DL + LL ± WLX)
18. 1.2(DL + LL ± WLY)

DL=dead load LL-live load

EQX=earthquake in X-direction

EQY=earthquake in Y-direction

WLX=wind load in X-direction

WLY=wind load in Y-direction

6.5 Building Models in E-tabs

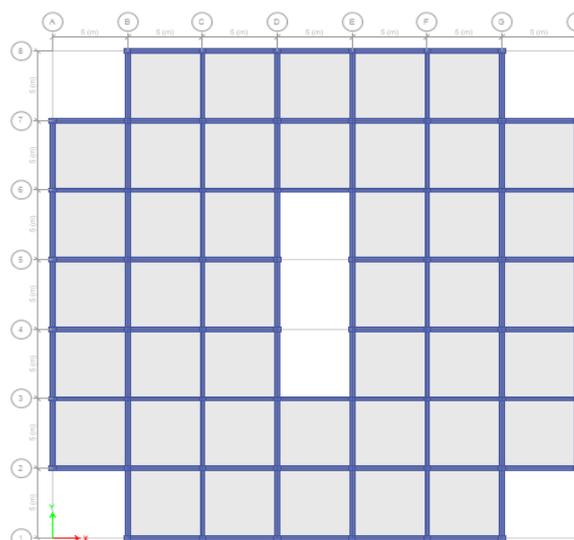


Figure 6.1: Plan view of residential building

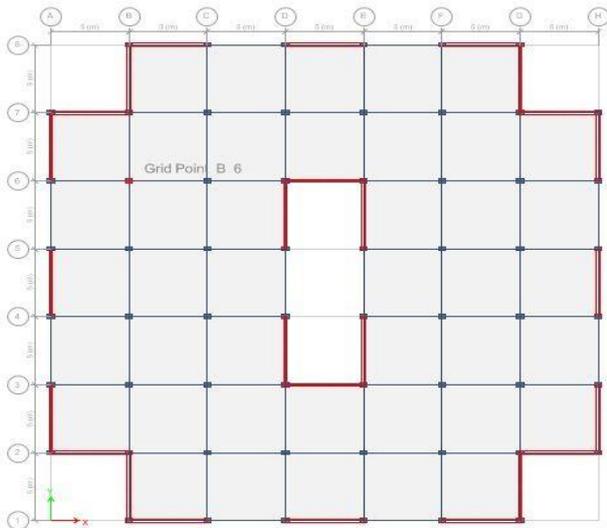


Figure 6.2: Plan view of high rise building with SHEARWALL

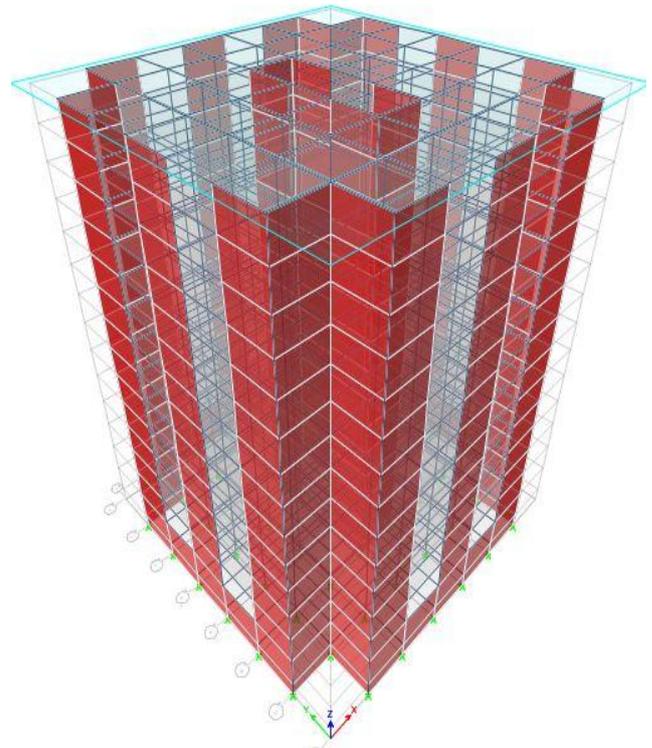


Figure 6.4: 3D view of high rise building with SHEARWALL

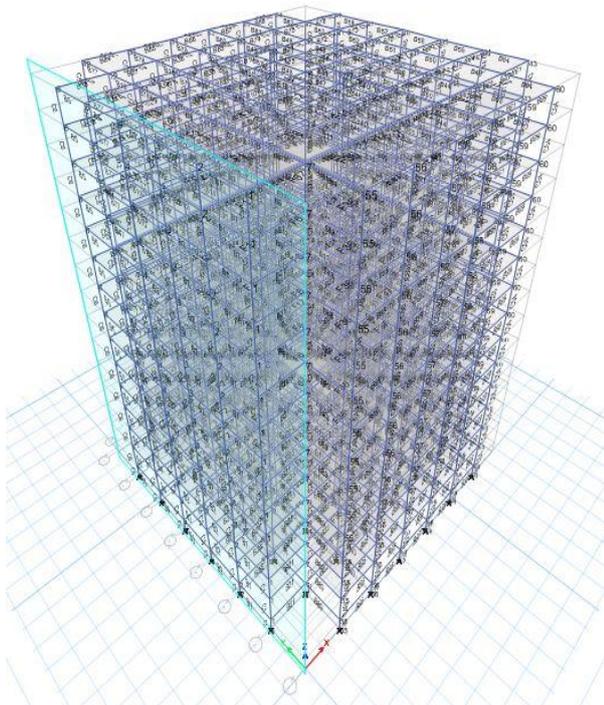


Figure 6.3: 3D view of high rise building without SHEARWALL

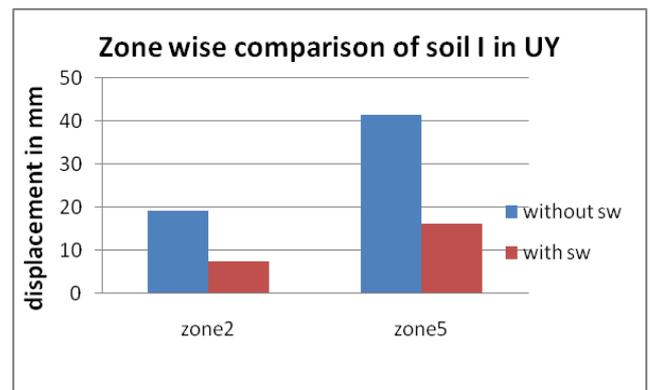
7. Results & Discussions

7.1 Study on Displacement

Sw-shear wall

Table 7.1: Comparison values of max displacement for Zones in soil-I in static analysis

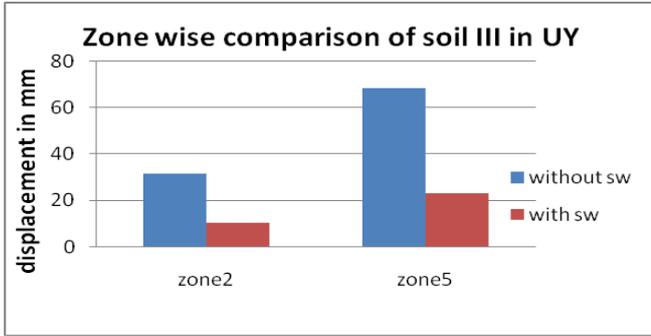
zones	soil-I	
	without sw	with sw
zone2	19.085mm	7.49mm
zone5	41.17mm	16.124mm



Graph 7.1: Displacements vs zones in soil-I in static analysis

Table 7.2: Comparison values of max displacement for Zones in soil-III in static analysis

zones	soil-III	
	without sw	with sw
zone2	31.84mm	10.78mm
zone5	68.7435mm	23.22mm

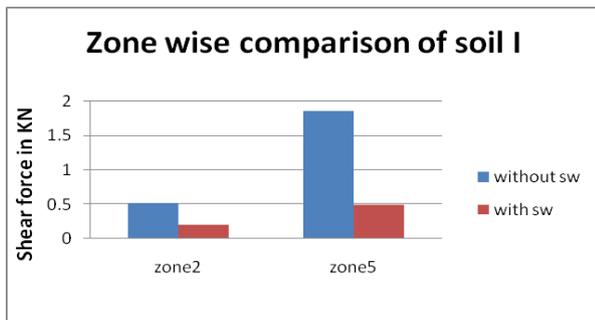


Graph 7.2: Displacements vs zones in soil-III in static analysis

7.2 Study on Shear force

Table 7.3: Comparison values of max Shear force for Zones in soil-I, dynamic analysis

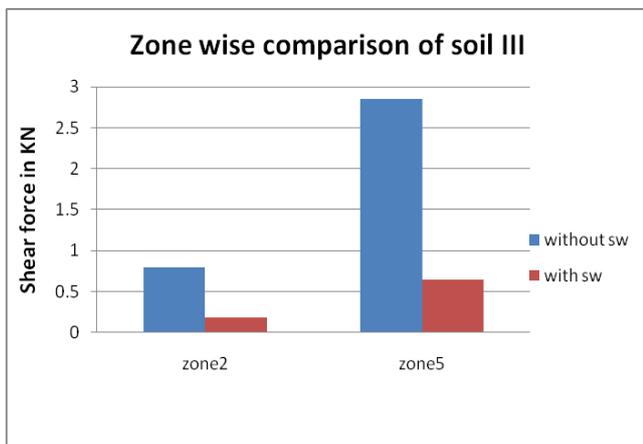
zones	soil-I	
	without sw	with sw
zone2	0.5174	0.2043
zone5	1.8625	0.491



Graph 7.3: Shear force vs zones in soil-I, dynamic analysis

Table 7.4: Comparison values of max Shear force for Zones in soil-III, dynamic analysis

zones	soil-III	
	without sw	with sw
zone2	0.7904	0.1804
zone5	2.8454	0.6495

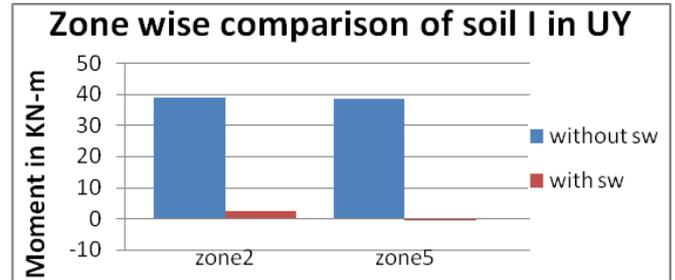


Graph 7.4: Shear force vs zones in soil-III, dynamic analysis

7.3 Study on Moment

Table 7.5: Comparison values of max Moment for Zones in soil-I in static analysis

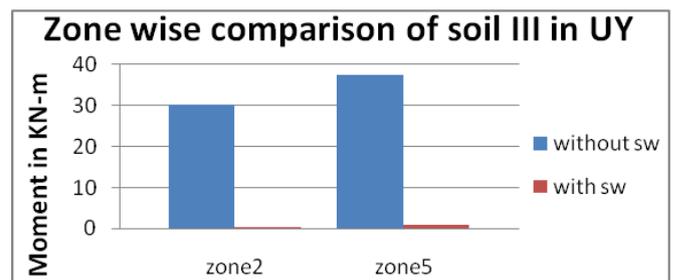
zones	soil-I	
	without sw	with sw
zone2	38.8891	2.5355
zone5	38.2442	-0.695



Graph 7.5: Moment vs zones in soil-I under Y- direction

Table 7.6: Comparison values of max Moment for Zones in soil-III in static analysis

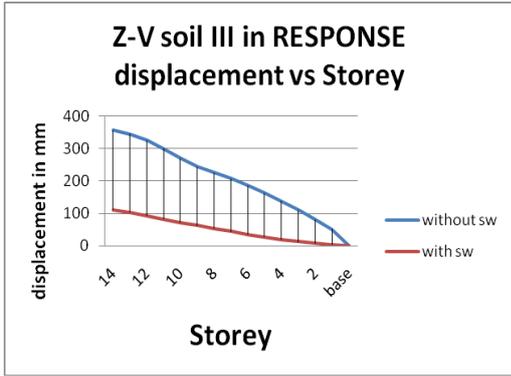
zones	soil-III	
	without sw	with sw
zone2	30.1219	0.2845
zone5	37.4396	0.8284



Graph 7.6: Moment vs zones in soil-III under Y- direction

Table 7.7: Displacement values for Zone V soil-III in Dynamic analysis

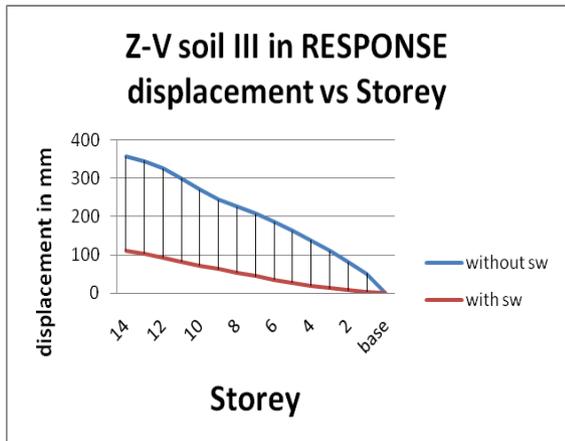
Storey	zone v soil III	
	Displacement in mm	
	without sw	with sw
14	356.1	111.5
13	344.3	102.4
12	325.1	92.7
11	299.3	82.9
10	269.6	73
9	243	63
8	225.2	53.7
7	206	44.3
6	184.8	35.3
5	161.7	26.9
4	136.6	19.1
3	109.7	12.4
2	81	6.9
1	50.3	3
base	0	0



Graph 7.7: Displacement vs Storey for Zone V soil-III in Dynamic analysis

Table 7.8: Displacement values for Zone V soil-III in Dynamic analysis

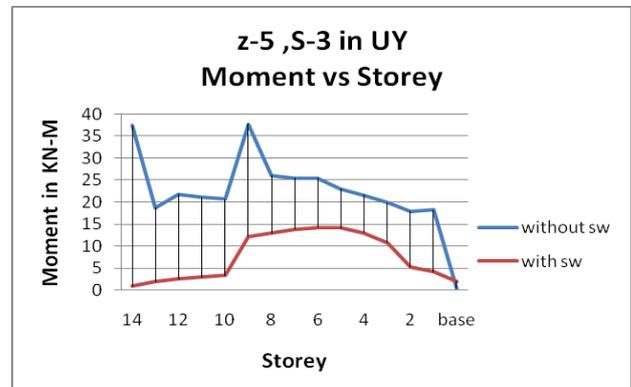
zone v soil III		
Storey	Displacement in mm	
	without sw	with sw
14	356.1	111.5
13	344.3	102.4
12	325.1	92.7
11	299.3	82.9
10	269.6	73
9	243	63
8	225.2	53.7
7	206	44.3
6	184.8	35.3
5	161.7	26.9
4	136.6	19.1
3	109.7	12.4
2	81	6.9
1	50.3	3
base	0	0



Graph 7.8: Displacement vs Storey for Zone V soil-III in Dynamic analysis

Table 7.9: Moment values for Zone V soil-III in static analysis

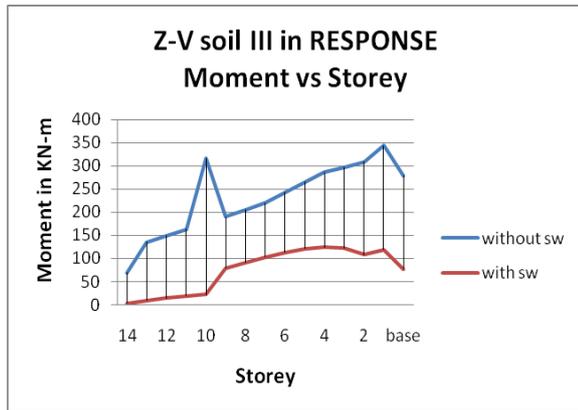
zone v soil III		
Storey	Moment in KN-m	
	without sw	with sw
14	37.4396	0.8284
13	18.7273	1.8486
12	21.7692	2.4619
11	21.083	2.8755
10	20.696	3.2783
9	37.7411	12.223
8	26.136	12.956
7	25.5448	13.793
6	25.464	14.239
5	23.108	14.18
4	21.6418	12.991
3	19.9088	10.828
2	17.934	5.2061
1	18.237	4.0658
base	0.2942	1.9363



Graph 7.9: Moment vs Storey for Zone V soil-III in static analysis

Table 7.10: Moment values for Zone V soil-III in Dynamic analysis

zone v soil III		
Storey	Moment in KN-m	
	without sw	with sw
14	69.5269	3.3399
13	135.3402	9.8739
12	149.1162	14.627
11	162.7367	19.404
10	315.3898	22.268
9	190.02	79.083
8	205.0293	91.105
7	220.4989	102.85
6	242.7754	113.11
5	265.242	121.03
4	286.201	124.48
3	296.8164	121.76
2	308.803	107.78
1	344.2328	119.50
base	279.4494	77.066



Graph 7.10: Moments vs Storey for Zone V soil-III in Dynamic analysis

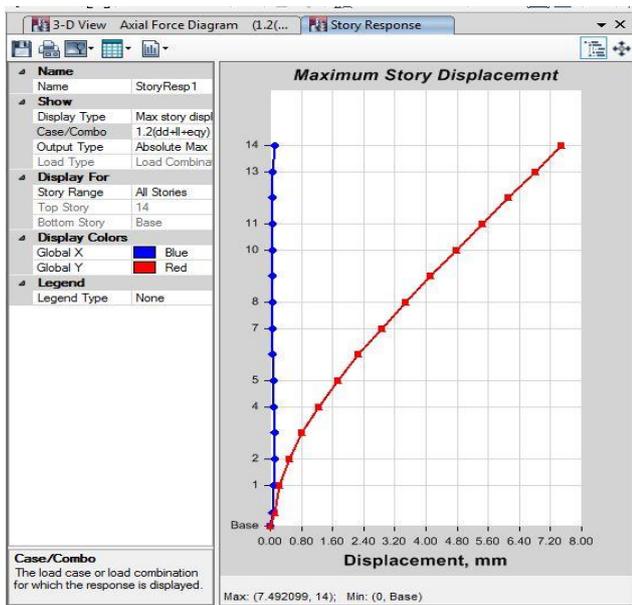


Figure 7.11: Showing displacement variation along Y

8. Discussions

Case-1: Variation of Displacement for different zones & soils:

In this case the reduction of Displacement is observed when the lateral systems i.e. when shear wall are provided in both directions UX & UY. The displacement for 14 storey building along UX direction is compared with Zones (Zone-II & Zone-V) is considered with soil I & soil III types s i.e. zone factor on X axis & displacement on Y axis, is to be noted that displacement of 40% is reduced from Z-2 to Z-5. Displacement in Y direction from it is to be noted that displacement of 40 % is reduced from Z-2 to Z-5.

Case-2: Variation of Shear for different Zones & Soils:

In this case the reduction of Shear is observed when the lateral systems i.e. when shear wall under static load for both directions UX & UY. The Storey Shear for 14 storey building along UX direction is compared with Zones (Zone-II & Zone-V) is considered with soil I & soil III types i.e. zone factor on X axis & Storey Shear on Y axis, from is to be noted that Storey Shear of 35% is reduced from Z-2 to Z-5. Storey Shear in Y direction it is to be noted that Storey Shear of 45% is reduced from Z-2 to Z-5.

Case-3: Variation of Moment for different Zones & Soils:

In this case Moment is analyzed in dynamic load and it is observed that Moment is increases with increase in zone factor. Taking different Zones (Zone-II & Zone-V) is considered with soil I & soil III types on X- axis & Y-axis and analyzed for each zone from Graph it is to be noted that Moment is decreased 40% from Z-2 to Z-5. Moment along Y from it is to be noted that Moment is increased 46% from Z-2 to Z-5.

Summary:

In this the results obtained from the package ETABS 2013 i.e. displacement, shear, moment, base shear & Moment are shown in graphs with different zones & different soil and the behavior of this are discussed in this chapter.

9. Conclusion

The following conclusions are made from the present study:

- 1) The center of mass and center of rigidity is influenced by adding and positioning of shear wall. It can be concluded that all models are symmetric about x-direction and there is no effect of torsion due to center of mass and center of rigidity in x-direction. The performance of structure with shear wall is better than structure without shear wall because center of mass and center of rigidity become closer.
- 2) Provision of shear wall generally results in reducing the displacement because the shear wall increases the stiffness of building and sustains the lateral forces. The better performance is observed and displacement is reduced in both x and y directions and shows better performances with respect to displacement when analysis is carried out by using response spectrum method.
- 3) The shear force resisted by the column frame is decreasing by placing the shear wall and the shear force resisted by the shear wall is increasing. This can be concluded indirectly by observing the maximum column shear force and moment in both directions. The moment resisting frame with shear walls are very good in lateral force such as earthquake and wind force. The shear walls provide lateral load distribution by transferring the wind and earthquake loads to the foundation. And also impact on the lateral stiffness of system and also carry gravity loads.
- 4) It is evident that shear walls which are provided from foundation to the roof top, are one of the excellent mean for providing earthquake resistance in high rise buildings. These are little expensive but desirable for safe structure.

References

- [1] IS: 456-code of practice for plain and reinforced concrete
- [2] IS: 875(part 1-5) - code of practice for structural safety of Building loading standards
- [3] IS 1893(Part-1):2002, Criteria for earthquake resistant design of structures.
- [4] IS 13920:1993, Ductile detailing of reinforced concrete structure subjected to seismic forces- code of practice.

- [5] SP: 16-design aids for reinforced concrete
- [6] Earthquake resistant design by pankaj agarwal.
- [7] Rosinblueth and Holtz “Analysis of shear walls in tall buildings” (1960)
- [8] Clough.R, King I.P and Wilson E.I-“Structural analysis of multi storied buildings” (1964)

Author Profile



N. Janardhana Reddy received the B. Tech Degree in Civil Engineering from Jawaharlal Nehru Institute of Technology (Ananatapur) in 2009 to 2013 . pursuing M. Tech specialization computer aided structural engineering in Bheema Institute of Technology Adoni 516501 in Kurnool dist.

Table 3: Histopathological Findings

Age	Histopathological Findings																				
	Fatty changes			Cirrhosis			Congestion			Hepatitis			Malignancy			Others			Normal		
	M	F	T	M	F	T	M	F	T	M	F	T	M	F	T	M	F	T	M	F	T
0-30	13	8	21	2	0	2	0	0	0	1	2	3	0	1	1	7	8	15	39	16	55
31-40	16	3	19	4	0	4	4	3	7	9	0	9	0	0	0	7	0	7	1	3	4
41-50	21	5	26	5	0	5	29	2	31	1	0	1	0	0	0	4	3	7	0	0	0
51-60	22	7	29	2	0	2	4	1	5	9	1	10	0	0	0	11	0	11	2	0	2
61-70	12	2	14	6	0	6	15	3	18	8	2	10	1	0	1	4	1	5	0	0	0
71-80	2	1	3	0	0	0	4	0	4	7	0	7	0	0	0	1	0	1	0	0	0
81-90	1	1	2	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
91-100	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	87	28	115	19	0	19	57	9	66	35	5	40	1	1	2	34	12	46	42	19	61
	32.95%			5.44%			18.91%			11.46%			0.57%			13.18%			17.48%		